

Structural Reliability Analysis of Limit State Functions With Multiple Design Points Using Evolutionary Strategies

Análisis de confiabilidad estructural de funciones de estado límite con múltiples puntos de diseño usando estrategias evolutivas

F. Barranco-Cicilia

*Mexican Petroleum Institute,
Research Program on Exploitation of Oil Fields in Deepwater, México DF.
E-mail: fbarran@imp.mx*

E. Castro-Prates de Lima

*COPPE-Federal University of Rio de Janeiro, Civil Engineering Department,
Cidade Universitária, Centro de Tecnologia, Brazil.
E-mail: edison@coc.ufrj.br*

L.V. Sudati-Sagrilo

*COPPE-Federal University of Rio de Janeiro, Civil Engineering Department,
Cidade Universitária, Centro de Tecnologia, Brazil.
E-mail: sagrilo@coc.ufrj.br*

(Recibido: abril de 2006; reevaluado: febrero de 2007; aceptado: agosto de 2008)

Abstract

The complete identification of all relevant design points is of paramount importance for the reliability analysis of real structures. In this paper a methodology based on Evolutionary Strategies (ES) algorithm is proposed to perform structural reliability analysis of limit state functions with multiple design points. A multidimensional optimization method using ES is first used to obtain a preliminary mapping of the relevant design points (local maxima). The approximate coordinates of these points are employed as initial guesses in the HL-RF (Hasofer and Lind–Rackwitz and Fiessler) algorithm when the First Order Reliability Method (FORM) for series systems is used to evaluate the system probability of failure. These points are also used as the central points when Monte Carlo Simulation with Importance Sampling (MCIS) method is employed in the structural reliability evaluation. Numerical applications show the feasibility and the robustness of the proposed methodology.

Keywords: System reliability analysis, multiple design points, evolutionary strategies, FORM, Monte Carlo simulation, importance sampling.

Resumen

La identificación de todos los puntos de diseño es de suma importancia en el análisis de confiabilidad de estructuras reales. En este artículo se propone una metodología basada en el algoritmo numérico de las Estrategias Evolutivas (ES) para llevar a cabo el análisis de confiabilidad estructural de funciones de estado límite con múltiples puntos de diseño. Inicialmente, un método de optimización multidimensional utilizando ES se aplica para obtener la posición aproximada de los puntos de diseño (máximos locales). Las coordenadas de estos puntos son utilizados como puntos de partida en el bien conocido algoritmo HL-RF (Hasofer y Lind – Rackwitz y Fiessler) para obtener la probabilidad de falla del

sistema con el método de confiabilidad de primer orden (FORM). Estos puntos también son utilizados como los centros para la evaluación de la probabilidad de falla a través del método de simulación numérica Monte Carlo con muestreo por importancia (MCIS). Aplicaciones numéricas muestran la versatilidad y la precisión de la metodología propuesta.

Descritores: Análisis de confiabilidad de sistemas, múltiples puntos de diseño, estrategias evolutivas, FORM, simulación Monte Carlo, muestreo por importancia.

Introduction

Randomness in loads, resistances and analytical models, causes the existence of a probability that structures do not meet the code standards used for their design. This probability is known as probability of failure (pf) and it can be evaluated mathematically through the next multiple integral:

$$pf = P[G(X) \leq 0] = \int_{G(X) \leq 0} \dots \int_{-\infty}^{\infty} f_X(x) dx, \quad (1)$$

where $f_X(X)$ is the joint Probability Density Function (PDF) of the n basic random variables $X = \{X_1, X_2, \dots, X_n\}^T$ into the limit state function $G(X)$. The limit state function is defined in such way that $G(X)=0$ separates the failure ($G(X) \leq 0$) and safe ($G(X) > 0$) domains.

Evaluation of equation (1) is not an easy task because it involves an n -fold integral over a complex domain. Various simulation-based and analytical methods have been proposed to deal with this problem. An approximation to pf can be obtained by analytical techniques such as First or Second Order Reliability Methods (FORM or SORM). The main idea of these methods is to move the reliability problem from the space of the basic random variables X to the space of standard normal statistically independent random variables U using a suitable transformation $U = T(X)$, such as Roseblatt or Nataf transformations (Melchers, 2001). In the U space, equation (1) can be expressed as:

$$pf = \int_{g(U) \leq 0} \dots \int_{-\infty}^{\infty} f_U(u) du = \int_{g(U) \leq 0} \prod_{i=1}^n \phi(u_i) du_i, \quad (2)$$

where $\phi(u_i)$ is the marginal PDF of a standard normal random variable U_i .

In FORM an approximation to the probability of failure is obtained by making the failure surface $G(U)=0$ linear at the design point U^* . This is the point on the failure surface closest to the origin and with the

highest probability (local maximum) in the failure domain of the standard normal space. The distance from the origin to the design point is the well-known reliability index $\beta = \|U^*\|$. Using the reliability index, pf is evaluated as:

$$pf = 1 - \Phi(\beta), \quad (3)$$

where $\Phi(\cdot)$ is the Cumulative Probability Function (CPF) of a standard normal random variable.

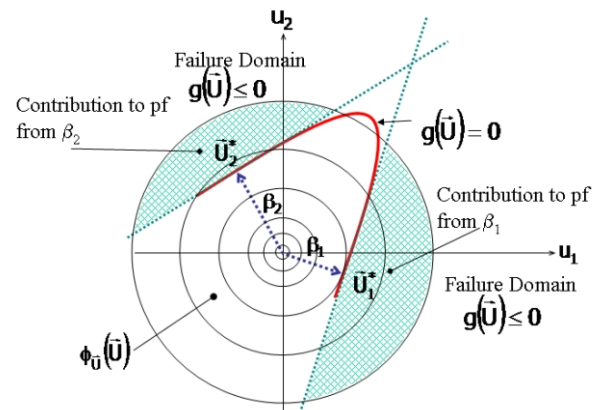


Figure 1. Limit state function with two design points

Generally, the FORM approximation gives a reasonable result for a limit state function with only one global design point. However, this is not the case when there are other local design points on the failure surface. A failure function with two design points is shown in figure 1. In this case, both design points have important contributions to the total system probability of failure and significant errors will be induced in it if one of them is missing. Unfortunately, the optimization algorithms used in connection with FORM, such as HL-RF approach, are only able to identify just one design point without giving any further information about the possibility of remaining design points. Multiple design points are also found in structural series system

problems where the global structural failure occurs when at least one of various limit states is violated (Melchers, 2001).

Monte Carlo simulation methods have also been largely employed to structural reliability analysis. In these methods N_S samples $u_i, i = 1, \dots, N_S$, are generated according with

$$f_U(u),$$

and then it is verified if the structure fails or not for each one of them. The pf is estimated as the number of failures divided by N_S . Since Monte Carlo method is basically a sampling process, the results are subjected to sampling error that decreases with the sample size. However, using procedures known as variance reduction techniques the error may be reduced without increasing the sample size. One of such procedures with a high convergence rate is the Monte Carlo with Importance Sampling (MCIS) (Melchers, 2001). In MCIS, the regions of interest for the simulation process are those around the points in the failure domain having the largest values for

$f_U(u)$, i.e., the design points U_i^* .

Over the last years few research studies on procedures for searching multiple design points have been published. For instance, Der Kiureghian *et al.*, (1998) presented an heuristic method, based on the HL-RF algorithm, for the systematic identification of multiple design points. In that method, each time that one design point is identified, the failure surface is deformed around this point through a bulge and the HL-RF method is re-initialized in order to search for another possible remaining design point. On the other hand, in the field of nonlinear optimization the so-called Genetic Algorithms have recently gained more attention to solve complicated problems (Michalewicz, 1992; Lagaros *et al.*, 2002). One important aspect associated to these algorithms is that they are not gradient-based methods as, for instance, the HL-RF approach used in reliability analysis.

In this paper the Evolutionary Strategies (ES) algorithm, a class of Genetic Algorithm, is employed to develop a new search methodology that is able to identify the presence of multiple design points in structural reliability analysis. The proposed methodology is applied in connection with FORM and/or MCIS to evaluate the probability of failure. Two numerical applications are presented: the first one is a non-linear limit state

function with two design points, and the second one is a limit state function with a series system representing failure mechanisms for a plane frame. The results obtained show the robustness and accuracy of the proposed approach.

Reliability analysis of limit state functions with multiple design points

The proposed methodology to perform the reliability analysis of functions with multiple design points consists of the following main steps:

1. Mapping of the approximate position of the relevant design points (local maxima) on the integration domain through the ES algorithm;
2. Improving the precision of design points coordinates applying the HL-RF method for each local maximum point identified in Step 1 when FORM approach (for series systems) is chosen for computing the probability of failure (an schematic representation is shown in figure 2); and/or
3. Using directly the points identified in Step 1 as the center points for the simulation process when the probability of failure is evaluated by MCIS.

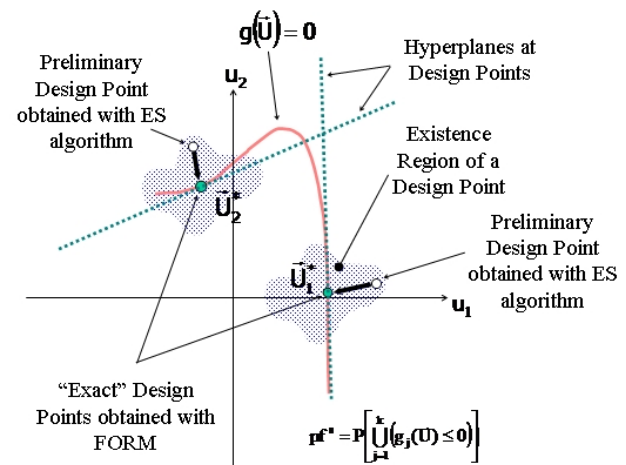


Figure 2. Schematic representation of the proposed methodology

Searching design points with evolutionary strategies

Finding the entire set of relevant design points (local maxima) is, in general, a very difficult problem to be solved by the classical gradient-based optimization

methods. The so-called Genetic Algorithms (Michalewicz, 1992) and Simulated Annealing Methods (Corana *et al.*, 1987) are much more effective and reliable to do this task because they have the capability to step up and out off regions near local maxima. Among the several Genetic Algorithms presented in literature there is the Evolutionary Strategies (ES) approach (Lagaros *et al.*, 2002; Greenwood, 1997). It is based on the principles of adaptive selection found in the natural world. Each generation (iteration of the algorithm) takes a population of individuals (potential solutions) and stochastically modifies the genetic material (problem parameters) to produce the new offspring.

For the application of the ES algorithm to structural reliability problems it is more convenient to extend the integration domain in equation (2) for all R^n space, using the following indicator function:

$$I[g(U) = 0] = \begin{cases} 1 & \text{if } g(U) \leq 0 \\ 0 & \text{if } g(U) > 0 \end{cases} \quad (4)$$

Using equation (4), equation (2) can be re-written as:

$$pf = \int_{R^n} \dots \int_{R^n} I[g(u) = 0] f_U(u) du \quad (5)$$

$$I[g(u) = 0] = \prod_{i=1}^n I_i(u_i) \quad (5)$$

Due to the rotational symmetry and exponential decay of the PDF in the normal standard space, the design points have the highest likelihood among all points in the failure domain (figure 3). Hence, the problem of finding these points can be solved by maximizing the argument of the integral in equation (5) in the R^n space. Thus, the design points' search process can be defined as the unrestricted optimization of the objective function $H(U)$:

$$\text{Maximize } H(U) = I[g(U) = 0] = \prod_{i=1}^n I_i(u_i) \quad (6)$$

The optimization problem in equation (6) can be solved through a simple version of the ES algorithm (Barranco, 2002). This algorithm is based on a population consisting of a single individual submitted to only mutation operations during his life. Two real vectors represent each individual: one containing a point in the search space U and other with the corresponding mutations $N(0, \sigma)$. The latter corresponds to a joint

statistically independent normal vector with each one of its components having zero mean and a standard deviation σ_i . The search algorithm is presented schematically in figure 3 and consists of the following steps:

1. Establish the lower and upper search limits, u_{a_i} and u_{b_i} , for each random variable u_i .
2. Set the first individual of the population. This initial point can be made equal to $U_0 = \{0, 0, \dots, 0\}^T$.
3. The objective function in equation (6), $H(U)_j$, is evaluated for each individual U_j . In this step, evaluation of the function $g(U_j)$ is required and then, transformation of the variables $X = T^{-1}(U)$ must be performed;
4. The offspring U_{j+1} of the individual U_j is generated by applying the following mutation operator:

$$U_{j+1} = U_j + N'(0, \sigma) \quad (7)$$

where $N'(0, \sigma)$ is one artificially generated realization of the random mutation vector $N(0, \sigma)$. A first estimative of $\sigma_i = (u_{a_i} - u_{b_i})/6$ can be assumed in order to extend the search field over all integration domain. During the subsequent simulations the standard deviations σ_i can be reduced in order to increase the numerical precision of the search algorithm;

5. The objective function $H(U)_j$ is evaluated for the new population member U_{j+1} . This offspring member will be only accepted in replacement of his progenitor if it satisfies all the problem constraints, and produces a better result for the objective function, i.e., $H(U_{j+1}) > H(U_j)$. If these conditions are not satisfied, the offspring is eliminated and its progenitor continues as member of the population;
6. The algorithm continues through steps 3 and 4 until a given maximum number of simulations is reached.

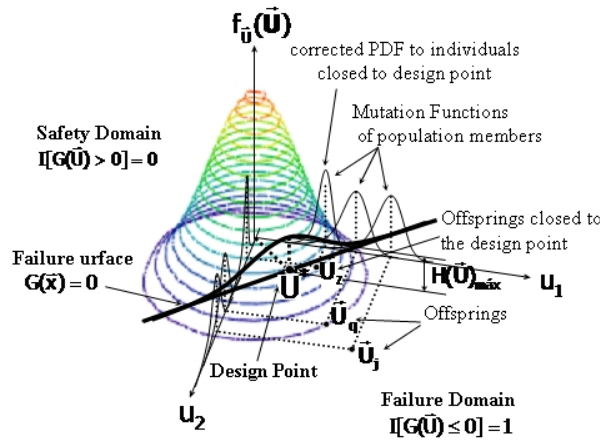


Figure 3. Evolutionary Strategies algorithm for searching the design points

The simulation-based procedure established in steps 1 to 6 above gives an approximation to just one design point. The algorithm is extended to find multiple design points by including constraints around the design points previously identified. Mathematically this is expressed by the updated optimization problem:

$$\begin{aligned} & \text{Maximize } H(U) = I[g(U) = 0] \prod_{i=1}^n U_i(u_i), \\ & \text{subject to: } \|U_j - U_k^*\| \geq R \text{ for all } k \end{aligned} \quad (8)$$

where U_k^* are all k -design points previously identified,

$$\|U_j - U_k^*\|$$

is the distance between the current point and k -th design point, and R is a chosen radius of a hyper-sphere in the R^n space.

In summary, after a design point has been found the remaining points are searched with the same algorithm now applied over the region of the R^n space outside the union of the hyper-spheres centered at the design points previously identified. Limited experience suggests a value in the range 1-3 units for R . Figure 4 shows the search space for a third design point, after U_1^* and U_2^* have been found. The algorithm is repeated until all the significant or a maximum specified number of design points in the integration domain have been found. The former condition is verified when the latest identified point U_k^* falls far away from the failure surface, i.e., $g(U_k^*) > 0.5$.

The algorithm presented above is easily adapted to consider series systems composed of M individual limit

state functions $g_i(U)$, $i=1, \dots, M$ (Madsen *et al.*, 1986). This is done by just changing the indicator function in equations (4) and (5) for $I[g_1(U) = 0 \dots g_M(U) = 0]$, where it is equal to one if at least one of the individual failure functions satisfies the condition $g_i(U) = 0$ and zero otherwise.

True design points using HL-RF method

Because the random nature of the search algorithm presented above, the precision of the results for the design points is dependent on the number of simulations. In order to increase the precision of the results without increasing the number of simulations the well-known HL-RF method (Melchers, 2001; Madsen *et al.*, 1986) can be used.

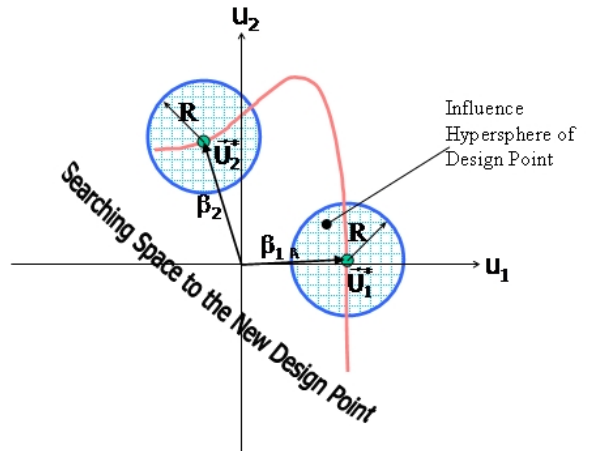


Figure 4. Search space for a new design point

This method is an optimization algorithm used in connection with FORM and SORM in the reliability analysis of structural systems. The objective of the HL-RF method is to obtain the design point U^* by solving the following constrained optimization problem in the standard normal random variables U space:

$$\min \|U\| \sqrt{U_1^2 \dots U_n^2}, \text{ subject to: } g(U) = 0 \quad (9)$$

The gradient-based solution technique of equation (9) consists in the generation of a sequence of points $U_i, i = 1, 2, 3, \dots$ according to the rule (Zhang *et al.*, 1994):

$$U_{i+1} = U_i + d_i \quad (10)$$

$$d_i = \frac{1}{\|g(U_i)\|^2} [U_i^T g(U_i) g(U_i)] g(U_i) - U_i$$

where d_i is a search direction vector, $g(U_i)$ is the gradient of the limit state function and U_i is the size of the increment. U_i is selected such as the inequality $m(U_{i+1}) - m(U_i)$ between the merit function:

$$m(U) = 0.5 \|U\|^2 + c \|g(U)\|$$

evaluated in two consecutive points is held. In the merit function, c is a parameter satisfying the condition

$$c = \|U\| / \|g(U)\|$$

in each step. U_{i+1} is considered as a design point U^* when the reliability index error in two consecutive iterations is less than or equal to an adequate tolerance margin, commonly in the order of 10^{-4} . In the present approach, each starting point U_0 in HL-RF technique is taken as one approximate design point identified by ES algorithm. The HL-RF is repeated as many times as the number of identified design points. The final set of design points is obtained by taking apart all repeated points, if they exist.

Evaluation of the probability of failure

FORM Approach

Under the FORM approach, the total failure probability associated to a series system or a single limit state function having multiple design points can be calculated through the unions and intersections of the failure domains associated to the hyper-planes tangent to each design point, as shown in figure 2. The probability of

failure is evaluated with the following equation (Madsen *et al.*, 1986):

$$pf = \sum_{j=1}^{N_p} P_j - \sum_{j=1}^{N_p} \sum_{i=1}^{N_p} P_{ji} + \sum_{j=1}^{N_p} \sum_{i=1}^{N_p} \sum_{l=1}^{N_p} P_{jil} - \dots \quad (11)$$

$$P_j = \Phi(-\beta_j) \text{ and } P_{ji} = \Phi(-\beta_j, -\beta_i, \rho_{ji})$$

where N_p is the number of elements of the system, β_j is the reliability index associated to the j -th design point, ρ_{ji} is the correlation coefficient between two hyper-planes, and $\Phi(\cdot, \cdot, \cdot)$ is the standard bi-normal CPF. Due to the fact that the individual failure probabilities P_j are generally small, the third order terms P_{jil} in equation (11) can be usually neglected.

The standard bi-normal CPF can be calculated with the equation (Madsen *et al.*, 1986):

$$\Phi(\beta_j, \beta_i, \rho_{ji}) = \int_0^{\beta_j} \int_0^{\beta_i} \phi(\beta_j, \beta_i, z) dz \quad (12)$$

$$\phi(\beta_j, \beta_i, z) = \frac{1}{2\sqrt{1-\rho_{ji}^2}} \exp \left\{ -\frac{1}{2} \frac{\beta_j^2 - 2\rho_{ji}\beta_j\beta_i + \beta_i^2}{1-\rho_{ji}^2} - \frac{z^2}{2} \right\}$$

where $\phi(\cdot, \cdot, z)$ is the standard bi-normal PDF.

Monte Carlo Simulation with Importance Sampling

Using Monte Carlo simulation with Importance Sampling (MCIS) technique for a structural reliability problem with multiple design points, the total probability of failure can be estimated by (Melchers, 2001)

$$pf = \int_{R^n} \dots I[g(u) = 0] \quad (13)$$

$$f_U(u) = \sum_{k=1}^{N_p} h_k^k(u) = \sum_{k=1}^{N_p} h_U^k(u) du$$

$$\text{where } h_U^k(u)$$

is a new sampling PDF and h_k is a weight factor. Numerically the integral in equation (13) can be estimated by

$$pf = \frac{1}{N_s} \sum_{i=1}^{N_s} I[g(u_i) = 0] f_U(u_i) = \sum_{i=1}^{N_s} h_U^k(u_i) \quad (14)$$

where N_s is the total number of samples (simulations) u_i generated artificially from the new sampling PDF. As it is shown by Melchers (2001), the k^{th} parcel of this function can be obtained by just shifting the peak of the joint PDF $f_U(u)$ to the design point:

$$U_k^* = (u_{1,k}^*, \dots, u_{n,k}^*), \text{ i.e.,} \quad (15)$$

$$h_U^k(u) = \prod_{i=1}^n (u_i - u_{i,k}^*)$$

and the k -th weight factor is giving by

$$w_k = \frac{f_U(U_k^*)}{\sum_{j=1}^{N_p} f_U(U_j^*)} \quad (16)$$

Standard formulae exist to evaluate the number of simulations N_s needed to achieve a desired level of confidence on the estimator given by equation (14) (Melchers, 2001). When compared to the crude Monte Carlo simulation approach the Importance Sampling technique greatly improves the convergence rate to achieve the failure probability, i.e., the number of simulation N_s is significantly reduced. It is important to notice that it is not necessary to employ the exact design points in equation (14) to take advantage of the MCIS technique. Then, the points identified by the ES algorithm described above can directly be used in connection with MCIS technique.

Numerical applications

The proposed methodology is applied initially to a case of bi-dimensional limit state function with multiple design points in order to illustrate some relevant aspects on convergence and precision of the proposed methodology. In the sequence, the reliability analysis of one plane frame structure having seven random variables is presented.

Example 1. Parabolic Limit State Function

The parabolic limit state function considered in this example is given by,

$$G(X) = 5.0 - X_2 - 0.5(X_1 - 0.1)^2 \quad (17)$$

where X_1 and X_2 are normal standard uncorrelated random variables with the characteristics presented in table 1. In this case, the original space X and the standard normal space U are the same. As pointed out by Der

Kiureghian *et al.* (1998), this failure function has two design points.

Table 1. probabilistic parameters of random variables for example 1

Random Variable	Distribution Type	Distribution Parameters	
		Mean	Std. Dev.
X_1	Normal	0.0	1.0
X_2	Normal	0.0	1.0

In the Evolutionary Strategy search algorithm, the origin of the standard normal space $U_0 = \{0,0,\dots,0\}$ was used as the first member of the population. After the first design point has been found, a hyper-sphere with radius equal to three units ($R=3$) was centered on it in order to constrain the search space for the next point and so on. To investigate the precision of the ES algorithm, the search for design points was carried out using different number of simulations, from 100 to 100,000. The ES approximations to design points are shown in figure 5, while figure 6 presents the corresponding errors in the approximated reliability indexes with respect to the ones obtained with FORM.

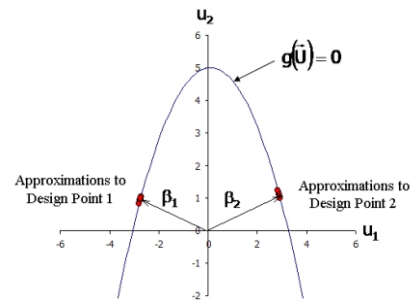


Figure 5. Approximations to the design points obtained with ES algorithm. Example 1

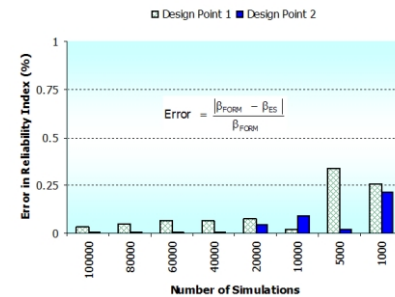


Figure 6. Errors in reliability indexes obtained with ES algorithm. Example 1

Figures 5 and 6 show that the search algorithm based on ES is very efficient to identify multiple design points. For example, for 10,000 simulations or more the error in reliability indexes obtained using only ES algorithm is less than 0.1% and with only 1,000 simulations the error is not more than 0.25%. Figure 7 presents the mean number of iterations that the HL-RF method used to find the design points, considering the guesses obtained with ES algorithm as starting points. It is noticed that even with a small number of 100 simulations, the mapping obtained by the ES is enough for the HL-RF method to identify the correct design points. It is also noticed that when the number of simulations increase the number of simulations in the HL-RF decreases, once the design points identified by the ES algorithm are very close to the correct ones as illustrated in figure 6.

Table 2 presents both the approximate design points obtained with ES (case of 10,000 simulations) and the ones using HL-RF method. The final design

points are the same as those published by Der Kiureghian *et al.* (1998).

In table 3 the total probability of failure is evaluated using the FORM approximation, considering isolated and joint contribution of each design point. Table 3 also includes the result from MCIS approach using the approximate design points obtained with ES and the exact ones obtained through numerical integration. The number of simulations for this latter approach has been calculated automatically in order to obtain a coefficient of variation (CoV) of 2.5% in the estimated probability of failure (Melchers, 2001).

It is observed that the system probability of failure computed by FORM taking into account only the contribution of the first design point presents an error of 39% and considering only the second point the error is around of 67%. However, when the contributions of both design points are considered the error is only of 6%. MCIS approach gives almost the exact probability of failure.

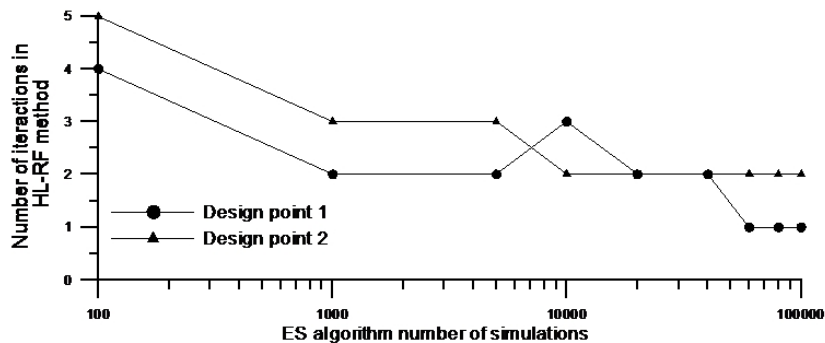


Figure 7. Number of iterations of HL-RF method to obtain true design points. Example 1

Table 2. ES and HL-RF design points. Example 1

Design Point	Evolutionary Strategies		HL-RF method			
	U_1	U_2	U_1	U_2	i	Iter.
1	-2.788	0.830	-2.741	0.965	2.906	3
2	2.934	0.984	2.916	1.035	3.094	2

Table 3. System failure probability. Example 1

Isolated U_1^* , FORM	Isolated U_2^* , FORM	U_1^* and U_2^* FORM	MCIS	Exact
0.00183	0.00099	0.00282	0.00304	0.00300

Example 2. Plane Frame Failure

This example considers the possibility of failure of the plane frame presented in figure 8 by means of plastic hinge mechanisms as investigated by Madsen *et al.* (1986). The failure function for this structure can be written by

$$G(X) = \min[g_1(X), g_2(X), g_3(X)] \quad (18)$$

which represents a series system of three failure mechanisms given by the following limit state functions:

$$g_1(X) = X_1 - X_2 - X_4 - X_5 - hX_6 \quad (19)$$

$$g_2(X) = X_1 - 2X_3 - 2X_4 - X_5 - hX_6 - hX_7$$

$$g_3(X) = X_1 - 2X_3 - X_4 - hX_7$$

In equation (18) $\min[.]$ means the minimum value of $[.]$. The characteristics of the statistically independent random variables are presented in table 4. It is known from Madsen *et al.* (1986) that equation (18) presents three design points, each one corresponding to a limit state function presented in equation (19). In (Madsen *et al.*, 1986) the design points are obtained, differently of the approach presented in this work, by separated reliability analyses for each limit state function $g_i(X)$.

By setting the maximum number of simulations equal to only 100 and $R=1$, the approximated design

points obtained by ES algorithm, and used as initial guesses for the HL-RF approach, are able to identify correctly the design points as shown in table 5. The results found for the probability of failure are compared with those from crude Monte Carlo simulation in table 6. As it can be observed the results obtained are in very good agreement.

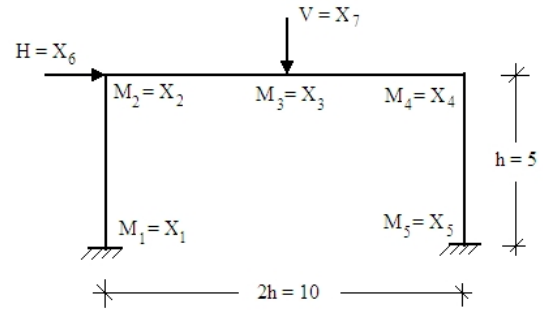


Figure 8. Plane frame structure (M_i node bending strength)

Table 4. Random variables properties for example 2

Random Variable	Distribution Type	Distribution parameters	
		Mean	Standard deviation
X_1, X_2, X_3, X_4, X_5	Lognormal	134.90	13.49
X_6	Lognormal	50.00	15.00
X_7	Lognormal	40.00	12.00

Table 5. Design points in the standard space for example 2

Random Variable	Proposed Methodology			Madsen <i>et al.</i> , 1986		
	Point 1	Point 2	Point 3	Point 1	Point 2	Point 3
X_1	-0.227	-0.221	0.000	-0.228	-0.222	0.000
X_2	-0.227	0.000	-0.290	-0.228	0.000	-0.289
X_3	0.000	-0.433	-0.564	0.000	-0.432	-0.564
X_4	-0.227	-0.433	-0.290	-0.228	-0.432	-0.289
X_5	-0.227	-0.221	0.000	-0.228	-0.222	0.000
X_6	2.673	2.391	0.000	2.672	2.382	0.000
X_7	0.000	1.456	3.366	0.000	1.466	3.368
	2.71	2.88	3.44	2.71	2.88	3.44

Table 6. System failure probability. Example 2

Reliability parameter	Proposed Methodology		Madsen <i>et al.</i> , 1986	Crude Monte Carlo Method
	FORM	MCIS	FORM	
P_f	0.004638	0.004772	0.004670	0.004783
E	2.602	2.592	2.601	2.591

As this problem is solved in R^7 space, it is not easy to find with high precision the design points directly through ES algorithm. In order to achieve a high precision, the number of simulations must be in the order 10^7 - 10^8 . However, using the ES algorithm in connection with HL-RF approach the number of simulations drops to the order of 10^2 .

Conclusions

The complete identification of all design points is of paramount importance for the reliability analysis of real structures due to the serious errors that can be introduced in the failure probability evaluation if any of them is neglected. In this paper a new simple and practical algorithm based on Evolutionary Strategies (ES) is presented to cope with the problem of multiple design points. Firstly, the ES algorithm is used to map over the integration domain the approximate position of the relevant design points (local maxima). Secondly, the coordinates of these points are used as initial guesses in the HL-RF algorithm to increase the numerical precision of the relevant design points coordinates. Finally, the structural failure probability can be evaluated by FORM approach for series systems or by Monte Carlo Simulation with Importance Sampling (MCIS) method using the identified design points as the center of the simulation process regions.

Trough the numerical examples presented in this work the ES algorithm showed to be a simple, very effective and reliable methodology for the identification of multiple design points. Depending on the number of simulations this algorithm can even identify precisely the position of the design points. This depends heavily on the dimension of the integration space, i.e., the number of random variables considered. However, its main advantage is the possibility of performing a systematic mapping of the number and position of all relevant design points on the integration domain using only a small number of simulations (around 100). With this rough mapping, the HL-RF approach easily arrives at the design points with a required precision.

One important point in the reliability analysis of some real structures is that the limit state functions usually cannot be expressed by means of analytical expressions and must be evaluated implicitly, for instance, through various finite element structural analyses. This aspect increases the computational costs of the analysis. To cope with this constraint an adaptive multidimensional interpolation approach, as presented by Barranco (2002) and Lima (1997), can be used to approximate analytically the true failure function. In this approach an initial set of interpolation points is successively updated in the interpolation scheme to encompass the regions associated with the maximum likelihood points. These points are quickly identified by the proposed search approach based on the approximate failure function.

References

- Barranco-Cicilia F. *Analysis of Systems with Multiple Design Points*. DSc Qualification Seminar. Department of Civil Engineering, COPPE/UFRJ, 2002 (In Portuguese)
- Corana A, Marchesi M, Martini C, Ridella S. Minimizing Multimodal Functions of Continuous Variables with the Simulated Annealing Algorithm. *ACM Transactions on Mathematics Software*, 13(3):262-280, 1987.
- Der-Kiureghian A, Dakessian T. Multiple Design Points in First and Second Order Reliability. *Structural Safety*, 20(1):37-49, 1998.
- Greenwood G.W. Chaotic Behavior in Evolution Strategies. *Physica D*, 109(3-4):201-241, 1997.
- Lagaros N.D., Papadrakakis M., Kokossalakis G., Structural Optimization Using Evolutionary Algorithms. *Computers and Structures*, 80(7-8): 571-589, 2002.
- Lima E.C.P. Structural Reliability Analysis by Unconstrained Optimization and Multimodal Monte Carlo Conditional Importance Sampling. In: Carneiro F.L.L., Ferrante A.J., Batista R.C. and Ebecken N.F.F., editors, 1997. International Offshore Engineering. Computational Mechanics Publications, Chichester.
- Madsen H.O., Krenk S., Lind N.S. *Methods of Structural Safety*. New Jersey. Prentice Hall. 1986.

Melchers R.E., *Structural Reliability Analysis and Prediction*. 2nd Edition. John Wiley. Chichester. 2001.
Michalewicz Z. *Genetic Algorithms+Data Structures=Evolution Programs*. Heidelberg. Springer-Verlag. 1992.

Zhang Y., Der-Kiureghian A. Two Improved Algorithms for Reliability Analysis. In: Proc. of the 6th IFIPWG 7.5 Working Conference on Reliability and Optimization of Structural Systems, Assisi, Italy, 1994.

About the authors

Federico Barranco-Cicilia. Graduated in Civil Engineering from the National Polytechnic Institute (Mexico) in 1992, he was granted a Master's Degree in Civil Engineering by the UNAM, in 1995, and a Doctor in Sciences degree by COPPE Federal University of Rio de Janeiro, with a thesis focused towards the technologies for hydrocarbon exploitation in deep waters. Dr. Barranco has published 6 technical articles in national congresses (Structural Engineering and Seismic Engineering), 8 in international congresses (OMAE, ISOPE and Rio Oil&Gas) and 2 in refereed international journals (ASCE and EE&SD). Since 1991, Dr Barranco works for the Mexican Petroleum Institute in the Research Program on Oil Field Exploitation in Deep Waters. Since 1992, he has lectured diverse courses on Structural Engineering at the Technological University of Mexico and lately at the Postgraduate Division of the Faculty of Engineering of the UNAM. Currently, Dr. Barranco is member of the National System of Researchers of the Research Council in Mexico, CONACyT.

Luís Volnei Sudati-Sagrilo. Received degrees in Civil Engineering (1986) from Federal University of Santa Maria (Brazil), Master in Sciences (1989) and Doctor in Sciences (1994) both from COPPE-Federal University of Rio de Janeiro (Brazil). He worked as a Research Engineer at the Department of Civil Engineering of COPPE/UFRJ from 1988 to 2005. In 2006 he became a Full Professor at the same department. His main research and academic activities are related to structural reliability, nonlinear random dynamic analysis and offshore structures. He has co-authored more than 75 technical publications and taken part in more than 50 consulting activities for private and state-owned companies in Brazil. He has already supervised 10 M.Sc. and 3 D.Sc. thesis. Recently, he has been pointed as the head of the Laboratory of Analysis and Reliability of Offshore Structures at COPPE/UFRJ.

Edison Castro-Prates de Lima. Received his graduation Diploma (1970) from Civil Engineering Department of UFRGS (Federal University of Rio Grande do Sul). Received his M.Sc. degree (1972) and D.Sc. degree (1977) from Civil Engineering Department of UFRJ (Federal University of Rio de Janeiro). Professor at Civil Engineering Department of COPPE/UFRJ, having had the following positions: 1972-1977, Assistant Professor; 1977-1983, Adjoin Professor and Full Professor since 1983. Most of his academic and research activities have been developed in structural engineering, finite element methods, random dynamic analysis, structural reliability, offshore structures, numerical methods and software development. He is co-author of over 175 scientific and technical publications, has supervised over 20 M.Sc. and 15 D.Sc. thesis. He is a scientific and technical adviser for several private and State companies and has supervised over